



BEHAVIOUR OF KESWICK CLAY UNDER TRIAXIAL COMPRESSION CONDITIONS

by

TAJURA GETACHEW OSSA

B.Sc. Engg.

(University of Addis Ababa, Ethiopia)

Thesis Submitted in Partial Fulfilment for the
Requirement of the Degree of
Master of Engineering Science

The University of Adelaide
Department of Civil and Environmental Engineering
Adelaide, South Australia

June, 1998

ABSTRACT

Expansive soils are characterised by total suction which is composed of matrix and osmotic suction whose changes result in a different stress-strain response. In the field, rain water, or gardening water; and in the laboratory triaxial tests use of distilled water as pore water changes the concentration and the type of ions in the soil pore water.

This research project investigates the relation between total suction and effective stress when applied simultaneously to the sample under triaxial conditions, and their effects on the measured shear strength and stress-strain behaviour of the samples. Consolidated, and unconsolidated triaxial tests, were conducted on "undisturbed" soil samples of Keswick Clay from Adelaide parklands. Tests included "high" pressure tests in which the cell pressure exceeded the in-situ total suction, as well as "low" pressure tests where the cell pressure was less than the in-situ total suction. Changes in total suction during the test were measured and compared to the applied effective stresses.

The results show that in UU tests, the change in total suction was depended on the initial total suction of the sample, the initial degree of saturation and the confining pressure. In CIU tests, the change in total suction was related with the change in effective stress and the initial total suction of the sample. The total suction measured after CIU triaxial tests was found to be directly proportional to the effective consolidation pressure. This linear trend was also found to apply for UU triaxial tests if the samples were allowed to reach uniform value of suction throughout the sample.

The undrained shear strength in UU triaxial tests, normalised with respect to either equivalent effective stress or total suction did not result in unique relationship, possibly due to stress history effects. Prediction of the in-situ overconsolidation pressure (OCR) using the modified Cam Clay model resulted in similar ratios to those found in previous tests on remoulded Keswick Clay. However, the variation of Bishop's pore pressure coefficients A_f , or A_r with OCR was found to lie above the established ranges of the experimental data. In CIU triaxial tests the undrained shear strength of the sample was directly proportional to both effective consolidation pressure and total suction of the samples after test.

TABLE OF CONTENTS

ACKNOWLEDGMENTS	i
STATEMENT OF ORIGINALITY	ii
ABSTRACT	iii
TABLE OF CONTENTS	iv
LIST OF FIGURES	x
LIST OF TABLES	xiv
CHAPTER 1	
INTRODUCTION	1
1.1 INTRODUCTION	1
1.2 OBJECTIVE AND SCOPE OF THE STUDY	3
1.3 THESIS LAYOUT	4
CHAPTER 2	
REVIEW OF THE BEHAVIOUR OF EXPANSIVE	
SOILS	5
2.1 INTRODUCTION	5
2.2 EXPANSIVE CLAY MINERAL PROPERTIES	7
2.2.1 Expansive clay minerals	7
2.2.2 Structural bond in expansive clay minerals	8
2.2.3 Water in clay minerals	10
2.3 IDENTIFICATION AND CLASSIFICATION OF EXPANSIVE	
SOILS	12
2.3.1 Expansive soil identification based on mineralogical	
composition	12
2.3.2 Expansive soil identification based on indirect methods.	13
2.3.2.1 Atterberg Limits	14
2.3.2.2 Soil suction	15

	2.3.2.3	Colloidal content	18
	2.3.2.4	Activity	18
	2.3.2.5	Specific surface area	19
2.3.3		Identification of expansive soils based on direct measurements	19
	2.3.3.1	Expansion index	20
	2.3.3.2	Swelling pressure.....	21
2.3.4		Classification of expansive soils	22
2.4		HEAVE PREDICTION OF EXPANSIVE SOILS	24
	2.4.1	Mechanics of swelling	25
	2.4.2	Prediction of expansive soil behaviour using mechanistic approaches.....	26
	2.4.2.1.	Oedometer tests	26
	2.4.2.2.	Instability index method	26
	2.4.2.3	Suction method.....	29
	2.4.2.4	Mathematical moisture flow model.....	31
2.5		EFFECTIVE STRESS APPROACH FOR PREDICTION OF EXPANSIVE SOIL BEHAVIOUR	32
	2.5.1	Effective stress in saturated soils.....	33
	2.5.2.	Effective stress in unsaturated soils.....	34
	2.5.3	Effective stress in expansive soils	35
	2.5.4	Stress state variables.....	36
	2.5.4.1	Shear strength of partially saturated soils in relation to state stress variables	36
	2.5.4.2	Volume change of partially saturated soils in relation to state stress variables.....	38
	2.5.5	Critical state model for unsaturated soil.....	40
2.6		FOUNDATION DESIGN ON EXPANSIVE SOILS	41
	2.6.1	Mound parameter prediction (e , y_m).....	43
	2.6.2	Design of slabs-on-ground on expansive soils	44
	2.5.2.1	Building Research Advisory Board (B.R.A.B.) method	44
	2.5.2.2	Lytton method.....	47

2.5.2.3	Walsh method	49
2.5.2.4	Mitchell method.....	51
2.5.2.5	Swinburne method.....	53
2.5.2.6	Probabilistic method.....	53
2.5.3	Summary of design methods	54
2.7	SUMMARY	55

CHAPTER 3 **EXPERIMENTAL PROGRAMME AND SOURCE OF SOIL**

3.1	INTRODUCTION.....	57
3.2	EXPERIMENTAL TESTING PROGRAMME.....	58
3.2.1	Triaxial testing.....	58
3.2.1.1	Consolidated undrained test (CIU)	59
3.2.1.2	Unconsolidated undrained test (UU)	61
3.2.2	Total suction	62
3.2.3	Preliminary index tests	63
3.3	SAMPLE RECOVERY AND HANDLING.....	63
3.4	APPARATUS AND PROCEDURES.....	67
3.4.1	Triaxial apparatus	67
3.4.2	Triaxial test Procedure.....	68
3.4.2.1	Sample preparation	68
3.4.2.2	Sample set-up	68
3.4.2.3	Sample loading	70
3.4.3	Transistor Psychrometer for suction measurement.....	70
3.4.3.1	The Transistor Psychrometer apparatus.....	71
3.4.3.2	Psychrometer calibration	73
3.4.3.3	Sample preparation for suction measurement	80
3.4.3.4	Measurement of total suction	80
3.4.4	Determination of soil indices.....	82
3.4.4.1	Sieve analysis	82
3.4.4.2	Clay content.....	84
3.4.4.3	Atterberg's Limits	85

	“high” pressure UU triaxial tests	104
4.3.2.2	Compressibility of soil samples subjected to “high” pressure UU triaxial tests	106
4.3.2.3	Effective stress paths of samples subjected to “high” pressure UU triaxial test conditions	108
4.3.3	Results of UU triaxial tests using soil water	109
4.4	RESULTS OF CIU TRIAXIAL TESTS	111
4.4.1	The initial conditions of the samples used in CU triaxial tests	112
4.4.2	Results of isotropic consolidation tests	113
4.4.3	Deviator stress of samples under consolidated undrained tests	114
4.4.4	Young’s moduli from consolidated undrained tests	120
4.4.5	Pore pressure coefficient from consolidated undrained tests	120
4.4.6	The effective stress path of samples under CU triaxial tests	121

CHAPTER 5	ANALYSIS AND DISCUSSION OF TEST RESULTS	125
5.1	INTRODUCTION	125
5.2	ANALYSIS OF TOTAL SUCTION CHANGES DURING TRIAXIAL TESTS	126
5.2.1	Analysis of total suction changes in consolidated undrained (CIU) triaxial tests	126
5.2.2	Change in total suction in UU tests	127
5.3	VERIFICATION OF TOTAL SUCTION CHANGES IN TRIAXIAL TESTS	128
5.3.1	UU triaxial test	128
5.3.2	CIU triaxial tests	131
5.4	RELATION BETWEEN EFFECTIVE STRESS AND TOTAL SUCTION	132

5.4.1	Relation between equivalent effective stress and initial total suction	132
5.4.2	Relation between effective consolidation pressure and total suction after test.....	137
5.5	ANALYSIS OF TRIAXIAL TEST RESULTS	139
5.5.1	Analysis of UU tests	139
5.5.1.1	Undrained shear strength	139
5.5.1.2	Undrained Young's modulus.....	142
5.5.2	Analysis of CIU tests	145
5.5.2.1	Undrained shear strength	145
5.5.2.2	Undrained Young's modulus.....	146
5.5.3	Application of Cam Clay model to the results	149
5.5.3.1	A_f versus OCR	150
5.5.3.2	Variation of normalised Young's modulus ((E_u/s_u), and (E_u/s'_c)) with OCR	150
5.5.3.3	Comparison of test results with published results ..	151
5.6	SIGNIFICANCE OF TOTAL SUCTION FOR TRIAXIAL TESTS	155
5.7	EFFECT OF TOTAL SUCTION AND EFFECTIVE CONSOLIDATION PRESSURE ON STRESS-STRAIN BEHAVIOUR DURING TRIAXIAL TESTS	156
5.8	SUMMARY	158
CHAPTER 6	SUMMARY AND CONCLUSIONS.....	160
6.1	INTRODUCTION.....	160
6.2	SUMMARY	160
6.3	RECOMMENDED FUTURE WORK.....	163
6.4	CONCLUSIONS.....	163
REFERENCES	165